


APOLLO SALES LTD PUBLIC ACCESS SCAFFOLD STEP DESIGN CHECK CALCULATIONS

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CALCULATION SHEET	Project : Apollo Public Access Tread			 ALAN WHITE DESIGN
	Element : Brief			
	Job Number : R0197	By : anw	Date:Feb13	
	Document No : 002	Checked :	Date:Feb13	

Brief

To carry out a design check on the Apollo Site Tread scaffold step to the relevant Standards and Codes.

Layout

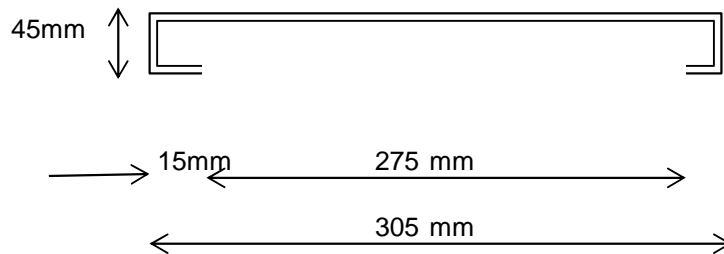


The step varies in length, supplied up to 1.5m wide tread

The step is 305mm wide with a non slip grating, non-see through grating.

Section


From BS 5395 Stairs Ladders and walkways the min going is 225mm



Loading

BS EN 1991-1-1 Table NA.3

UDL	w=	3.00 kN/m ²
Point load on 200 by 200mm	W=	4.00 kN

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Factor of safety From BS EN 12811-1

1.1.1.1 Partial safety factors for actions, gF

Except where stated otherwise, the partial safety factors, gF, shall be taken as follows:

Ultimate limit state

$\gamma_F = 1,5$ for all permanent and variable loads

$\gamma_F = 1,0$ for accidental loads

Serviceability limit state


$\gamma_F = 1,0$

10.3.2.2 Partial safety factors for resistance gM

For the calculation of the design values of the resistances of steel or aluminium components the partial safety factor, . For components

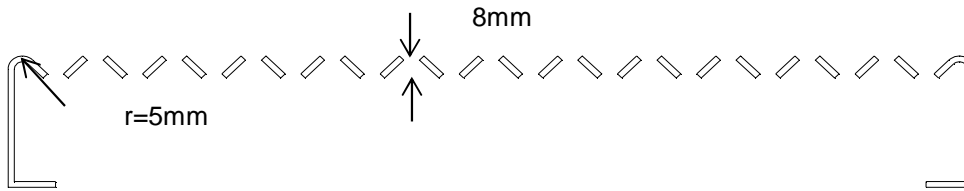
$\gamma_M=1,1$

of other materials the partial safety factor, gM, is to be taken from relevant standards.

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Section properties

From autocad massprop using the section as shown below
This is conservative as the section chosen is the minimum



A=	732 mm ²
I _x =	145250 mm ⁴
I _y =	9054649 mm ⁴
r _x =	14.1 mm
r _y =	111.2 mm
W _{pl,y} =	8236 mm ³

Slenderness	C/t=	291/8	C= 305-2*2-2*5 = 291mm t= 8mm
	=	36.38	
	<	38.00	

Section is class 2 so plastic design allowable

Material is mild steel f_y=275N/mm²

Moment capacity

From BS EN 1993-1-1- 6.2.5

$$M_{c,Rd} = W_{el} f_y / \gamma_{M0}$$

$$= 8.24 * 275 / 1100$$

$$= 2.06 \text{ kNm}$$

W_{el}= 8.24cm³
f_y= 275N/mm²
γ_{M0}= 1.1

Shear capacity

From BS EN 1993-1-1- 6.2.6

$$V_{c,Rd} = A_v (f_y / \sqrt{3}) / \gamma_{M0}$$

$$= 180 * (275 / \sqrt{3}) / 1100$$


$$= 25.98 \text{ kN}$$

A_v= 2*45*2
= 180mm²
f_y= 275N/mm²

Lateral Torsional Buckling

From BS EN 1993-1-1- 6.3.2.1

As bending is about minor axis LTB verification is not required.

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Loading

As before

UDL on stair w= 3.00 kN/m²
and point load W= 4.00 kN on 200mm by 200mm

Moment

so for UDL on 225mm wide stair with span of 1.6m

$$M_{Ed} = \gamma \cdot w \cdot B \cdot L^2 / 8 \qquad \gamma = 1.5$$

w= 3kN/m²
B= 0.305m
L= 1.5m

$$= 1.5 \cdot 3 \cdot 0.305 \cdot 1.5^2 / 8$$

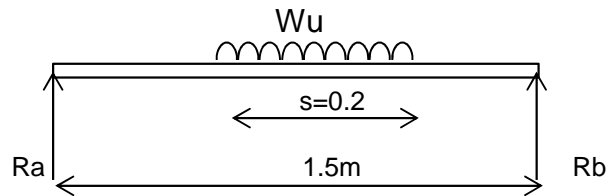
$$= \mathbf{0.39 \text{ kNm}}$$

for a point load of 4kN over a width of 200mm

$$W_u = \gamma W \qquad \gamma = 1.5$$

$$= 1.5 \cdot 4$$

$$= \mathbf{6.00 \text{ kN}}$$



$$R_a = W_u / 2 \qquad W_u = 6 \text{ kN}$$

$$= 6 / 2$$

$$= \mathbf{3.00}$$

$$M_{Ed} = R_a \cdot L / 2 - W_u \cdot s / 4$$

s= 0.2m
Wu= 6kN

$$= 3 \cdot 1.5 / 2 - 6 \cdot 0.2 / 4$$


$$= \mathbf{1.95 \text{ kNm}}$$

L= 1.5m

from previous calculations

$$M_{c,Rd} = \mathbf{2.06 \text{ kNm}}$$

$$> \mathbf{1.95} \qquad \mathbf{ok}$$

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Shear

so for UDL on 225mm wide stair with span of 1.6m

$$V_{Ed} = \gamma \cdot w \cdot B \cdot L / 2 \quad \gamma = 1.5$$

$$= 1.5 \cdot 1 \cdot 0.225 \cdot 1.6 / 2$$

$$= 0.27 \text{ kN}$$

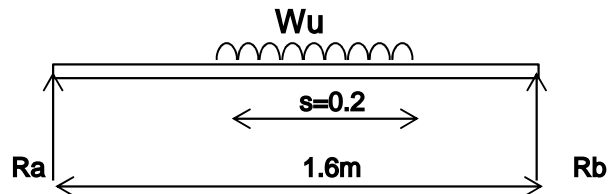
$w = 1 \text{ kN/m}^2$
 $B = 0.225 \text{ m}$
 $L = 1.6 \text{ m}$

for a point load of 4kN over a width of 200mm

$$W_u = \gamma W \quad \gamma = 1.5$$

$$= 1.5 \cdot 1.5$$

$$= 2.25 \text{ kN}$$



$$R_a = W_u / 2 \quad W_u = 2.25 \text{ kN}$$

$$= 2.25 / 2$$

$$= 1.13$$

$$V_{Ed} = R_a$$

$$= 1.13 \text{ kN}$$

from previous calculations

$$M_{c,Rd} = 25.98 \text{ kN}$$

$$> 1.13 \quad \text{ok}$$

Deflection

for central point load of 1.5kN

$$d = WL^3 / 48EI \quad W = 1.5 \text{ kN}$$

$$L = 1.6 \text{ m}$$

$$E = 205 \text{ kN/mm}^2$$

$$I = 123982 \text{ mm}^4$$

$$= 1500 \cdot 1600^3 / (48 \cdot 205 \cdot 10^3 \cdot 123982)$$

$$= 5.04 \text{ mm}$$


From BS EN 12811-1 the max deflection is

$$d = L / 100 \quad L = 1600 \text{ mm}$$

$$= 1600 / 100$$

$$= 16.00 \text{ mm}$$

$$> 5.04 \quad \text{ok}$$

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Rotation

If the loading was eccentric, ie at the step edge the moment applied to the support couplers would be

$$\begin{aligned}
M &= W_u \cdot l_a & W_u &= 2.25 \text{ kN} \\
& & l_a &= 0.225/2 \\
& & &= 0.11 \text{ m} \\
&= 2.25 \cdot 0.11 \\
&= 0.248 \text{ kNm}
\end{aligned}$$

This is conservative as the load is spread over a patch not a point.

this is resisted by two couplers so the twisting moment is

$$M = 0.124 \text{ kNm}$$

From BS EN 12811-1 the resistance to rotation of a double coupler is

$$\begin{aligned}
M_r &= 0.130 \text{ kNm} \\
&> 0.124 && \text{ok}
\end{aligned}$$

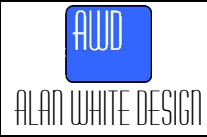
If the load is considered as a patch 200mm wide on a 225mm wide step then the eccentricity

$$\begin{aligned}
l_a &= 225/2 - 200/2 \\
&= 12.50 \text{ mm}
\end{aligned}$$

$$\begin{aligned}
\text{and } M &= W_u \cdot l_a \\
&= 2.25 \cdot 0.0125 \\
&= 0.03 \text{ kNm} \\
&<< 0.13 && \text{ok}
\end{aligned}$$

CALCULATION SHEET

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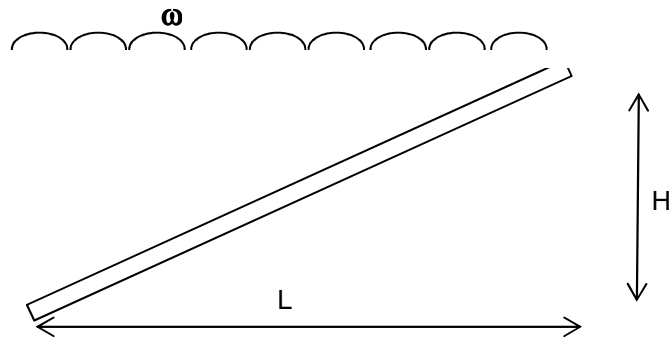


Length of stringer

From BS EN 12811-1, the load on the stringer is :

The structure of the stairways shall be capable of supporting a uniformly distributed load of 1,0 kN/m² on all treads and landings within a height of 10 m. but the load on the tread is 3kN/m

so the stringer will carry the load as shown below



where
for one stringer

$$\begin{aligned}
 w &= 3.0\text{kN/m}^2 * 1.5\text{m}/2 \\
 &= 3 * 1.5/2 \\
 &= \mathbf{2.25 \text{ kN/m unfactored}}
 \end{aligned}$$

and

L= length between stringer supports
H= height varies with angle between 30 and 55 deg

taking allowable values from TG20 as

Moment M= 1.1 kNm
Axial P= varies as below

Angle °	Moment kNm	Axial kN	Combined	Length m
30	0.84	3.37	1.00	2.59
35	0.83	3.79	1.00	2.40
40	0.81	4.17	1.00	2.21
45	0.80	4.51	1.00	2.00
50	0.79	4.82	1.00	1.80
55	0.78	5.09	1.00	1.59

Above table found by Excel Goal seeking

